







#### 38<sup>th</sup> ANNUAL GEOTECHNICAL SEMINAR

GEO-Omaha 2021

FEBRUARY 5, 2021 SCOTT CONFERENCE CENTER 6450 PINE STREET OMAHA, NEBRASKA

#### GEOTECHNICAL SEMINAR DEPARTMENT OF CIVIL ENGINEERING UNIVERSITY OF NEBRASKA OMAHA, NEBRASKA 68182

# Gabion-Faced Reinforced Embankment Case study and Installation Considerations

Date Friday, February 5, 2021

Author Stefano Rignanese M.Sc., Technical Marketing Manager NA



#### **TOPICS COVERED**





- MSE Wall Definition
- MSE Wall Classification
- Introducing Gabion-Faced Reinforced Embankments
- Case Study
- Installation Process
- Questions

#### MSE WALL DEFINITION



DEPARTMENT OF CIVIL ENGINEERING





#### 2.1.1 MSE Walls

MSEW structures are cost-effective alternatives for most applications where reinforced concrete or gravity type walls have traditionally been used to retain soil. These include bridge abutments and wing walls, as well as areas where the right-of-way is restricted, such that an embankment or excavation with stable side slopes cannot be constructed. They are particularly suited to economical construction in steep-sided terrain, in ground subject to slope instability, or in areas where foundation soils are poor.

MSE walls offer significant technical and cost advantages over conventional reinforced concrete retaining structures at sites with poor foundation conditions. In such cases, the elimination of costs for foundation improvements such as piles and pile caps, that may be required for support of conventional structures, have resulted in cost savings of greater than 50 percent on completed projects.

Representative uses of MSE walls for various applications are shown in Figure 2-1.

Temporary MSE wall structures have been especially cost-effective for temporary detours necessary for highway reconstruction projects. Temporary MSE walls are used to support temporary roadway embankments and temporary bridge abutments, as illustrated in Figure 2-2. MSE walls are also used as temporary support of permanent roadway embankments for phased construction, an example is shown in Figure 2-3.

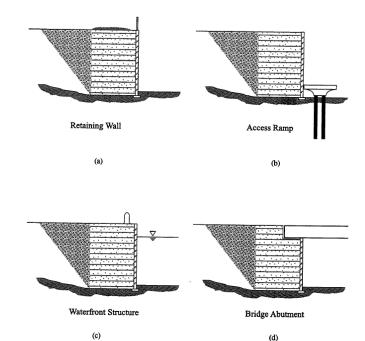


Figure 2-1. Representative MSE wall applications (a) retaining wall; (b) access ramp; (c) waterfront structure; and (d) bridge abutment.

#### MSE WALL ADVANTAGES AND POTENTIAL DISADVANTAGES





#### 2.2.1 Advantages of Mechanically Stabilized Earth (MSE) Walls

MSE walls have many advantages compared with conventional reinforced concrete and concrete gravity retaining walls. MSE walls:

- Use simple and rapid construction procedures and do not require as large of construction equipment.
- · Do not require special skills for construction.
- · Require less site preparation than other alternatives.
- · Need less space in front of the structure for construction operations.
- Reduce right-of-way acquisition.
- Do not need rigid, unyielding foundation support because MSE structures are tolerant to deformations.
- · Are cost effective.
- Are technically feasible to heights in excess of 100 ft (30 m).

Pre-manufactured materials, rapid construction, and, competition among different proprietary systems has resulted in a cost reduction relative to traditional types of retaining walls. MSE walls are likely to be more economical than other wall systems for walls higher than about 10 ft (3 m) or where special foundations would be required for a conventional wall.

One of the greatest advantages of MSE walls is their flexibility and capability to tolerate deformations due to poor subsoil foundation conditions. Also, based on observations in seismically active zones, these structures have demonstrated a higher resistance to seismic loading than rigid concrete wall structures.

#### 2.2.3 Potential Disadvantages

The following general potential disadvantages may be associated with all reinforced soil structures, and are dependent upon local and project conditions:

- Require a relatively large space (e.g., excavation if in a cut) behind the wall or slope face
  to install required reinforcement.
- MSE walls require the use of select granular fill. (At some sites, the cost of importing suitable fill material may render the system uneconomical.) Reinforced fill requirements for RSS are typically less restrictive.
- The design of soil-reinforced systems often requires a shared design responsibility between material suppliers and owners.

#### TYPES OF SYSTEMS

Nebraska Lincoln

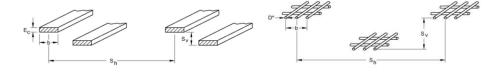




GEOTECHNICAL SEMINAR
DEPARTMENT OF CIVIL ENGINEERING
UNIVERSITY OF NEBRASKA
OMAHA, NEBRASKA 68182

#### Reinforcement Geometry

- Linear unidirectional. Strips, including smooth or ribbed steel strips, or coated geosynthetic strips over a load-carrying fiber.
- Composite unidirectional. Grids or bar mats characterized by grid spacing greater than 6 in. (150 mm).
- Planar bi-directional. Continuous sheets of geosynthetics, welded wire mesh, and woven wire mesh. The mesh is characterized by element spacing of less than 6 in. (150 mm).



#### **Reinforcement Material**

Reinforcement Material Distinction can be made between the characteristics of metallic and nonmetallic reinforcements:

- Metallic reinforcements. Typically of mild steel. The steel is usually galvanized.
- Nonmetallic reinforcements. Generally polymeric materials consisting of polyester or polyethylene.







Figure 17, Metallic Reinforcement (Inextensible). Left: metallic grid. Right: Ribbed Strips.

Source: FDOT MSEW INSPECTOR'S HANDBOOK

ParaWeb<sup>®</sup>

#### Reinforcement Extensibility

Reinforcement Extensibility There are two classes of extensibility relative to the soil's extensibility:

- Inextensible. The deformation of the reinforcement at failure is much less than the
  deformability of the soil. Steel strip and bar mat reinforcements are inextensible.
- Extensible. The deformation of the reinforcement at failure is comparable to or even
  greater than the deformability of the soil. Geogrid, geotextile, and woven steel wire mesh
  reinforcements are extensible

Source: FHWA NHI-10-024, MSE Walls and RSS – Vol I, Page 2-11







#### FACING SYSTEMS

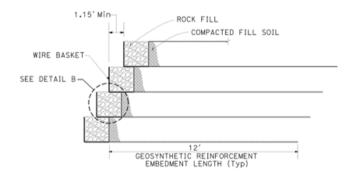




Segmental precast concrete panels

Dry cast modular block wall (MBW) units

**Gabion Facing** 







**Geosynthetic Facing** 

#### INTRODUCING GABION-FACED MSE WALL







Publication No. FHWA-NHI-10-024 FHWA GEC 011 - Volume I

#### NHI Courses No. 132042 and 132043

#### Design and Construction of Mechanically Stabilized Earth Walls and Reinforced Soil Slopes - Volume I

Developed following: AASHTO LRFD Bridge Design Specifications, 4th Edition, 2007,

AASHTO LRFD Bridge Construction Specifications, 2nd Edition, 2004, with 006, 2007, 2008, and 2009 Interims.

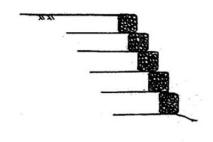












Gabion-Facing

Gabion Facing. Gabions (rock-filled wire baskets) can be used as MSE wall or RSS facing with reinforcing elements consisting of welded wire mesh, welded bar-mats, geogrids, geotextiles or the double-twisted woven mesh placed between or integrally manufactured with the gabion baskets. For example, this facing system is used by Maccaferri for their Terramesh® wall system.

Source: FHWA NHI-10-024, MSE Walls and RSS – Vol I, Page 2-15

## INTRODUCING GABION-FACED MSE WALL

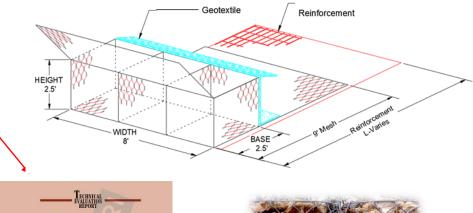
Nebraska Lincoln GEOTECHNICAL SEMINAR DEPARTMENT OF CIVIL ENGINEERING

G-I GEO-INSTITUTE Metrocks Chapt

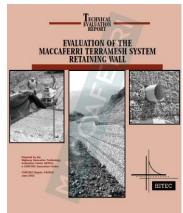
DEPARTMENT OF CIVIL ENGINEERING UNIVERSITY OF NEBRASKA OMAHA, NEBRASKA 68182

 Terramesh® System is a reinforced soil system with gabion facia and double twist wire mesh reinforcement

Durability: **up to 75 years** as certified by **HITEC** evaluation report













UNIVERSITY OF NEBRASKA

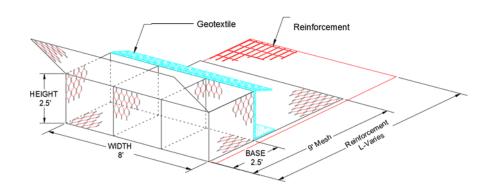




DT wire mesh unit

4" to 8" rock (100 mm - 200 mm)

**ASSHTO M288 Class 3 Geotextile** 



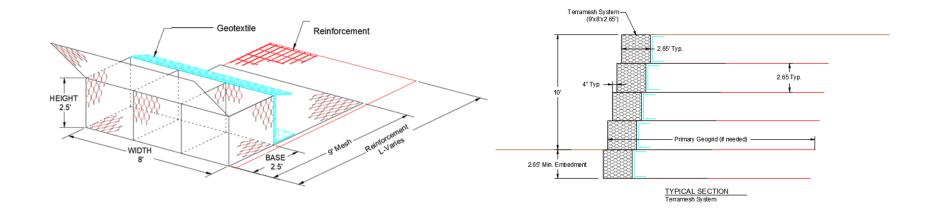








GEO I ECHNICAL SEMINAR DEPARTMENT OF CIVIL ENGINEERING UNIVERSITY OF NEBRASKA OMAHA, NEBRASKA 68182



Reinforcement Geometry PLANAR BI-DIRECTIONAL (DT MESH)

Reinforcement Material METALLIC REINFORCEMENT (DT MESH) + POLYMER COATING

Reinforcement Extensibility EXTENSIBLE REINFORCEMENT

Facing System GABION FACING

#### MATERIAL SENT TO THE JOBSITE





## Terramesh® System is a pre-fabricated unit



The units are sent to the jobsite ready to be installed!

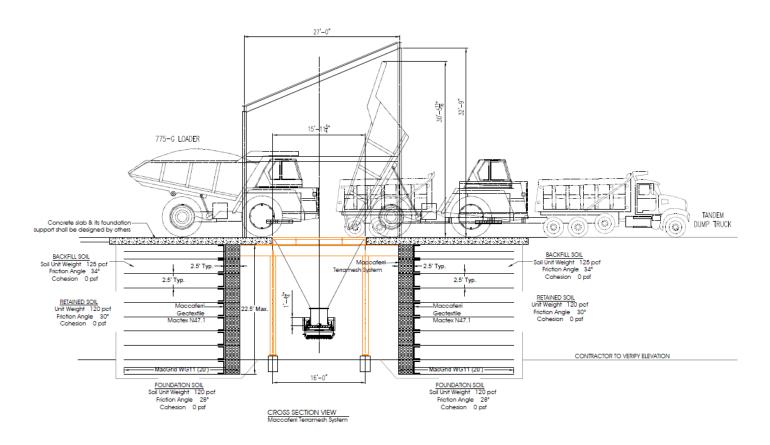


DEPARTMENT OF CIVIL ENGINEERING UNIVERSITY OF NEBRASKA OMAHA NEBRASKA 68182





#### CASE STUDY IN NEBRASKA – PROJECT PLANS

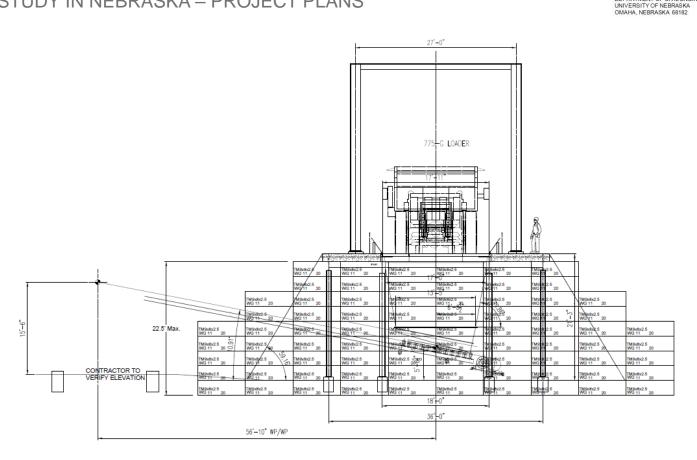








#### CASE STUDY IN NEBRASKA – PROJECT PLANS







#### CASE STUDY IN NEBRASKA - MATERIAL RECEIVED IN THE JOBSITE









## UNLOAD UNITS FROM THE BUNDLE







## CUT GEOGRID TO REQUIRED LENGTH (CHECK DRAWINGS)





## **CUT GEOTEXTILE**







3 rolls of 5'x300'

## **UNIT ASSEMBLY**











## **UNIT ASSEMBLY**

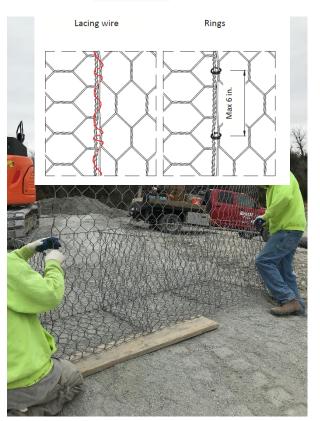


















## **ASSEMBLED UNIT**



#### **GEOGRID PLACEMENT**

















## UNIT PLACEMENT ON TOP OF THE GEOGRID











#### UNIT CONNECTION WITH ADJACENT UNITS













Fig. 10 - Placing of rings by pneumatic lacing gun to connect two adjacent wire mesh tails

## SMALL LAYER TO STOP POSSIBLE GEOGRID MOVEMENT





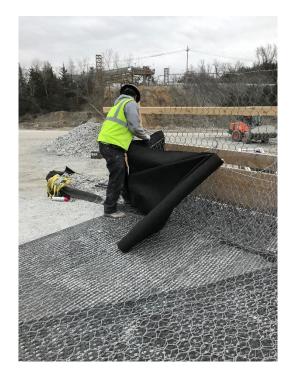




#### **GEOTEXTILE INSTALLATION**



G-1 Deposite Institute Refereda Chapter













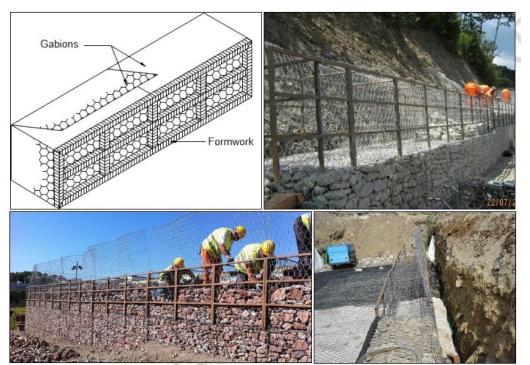




Fig. 16 - Use of a steel frame to avoid gabion bulging

## **ROCK INSTALLATION**



ASCE



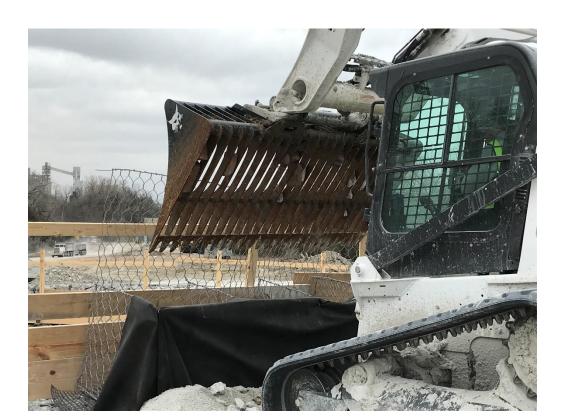




#### **ROCK INSTALLATION**







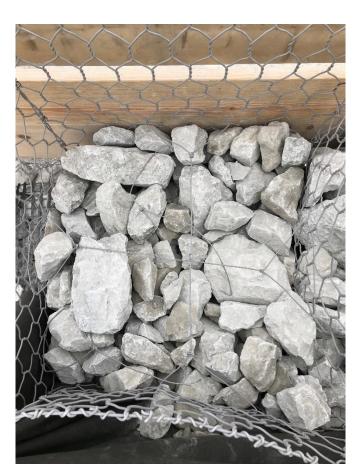
#### Rock

Rock used in the gabion fascia section of a Terramesh unit shall be hard, angular to round, durable and of such quality that they shall not disintegrate on exposure to water or weathering during the life of the structure. The rocks shall range between 4 in (100 mm) and 8 in (200 mm). Each range of sizes may allow for a variation of 5% oversize rock by number of particles, or 5% undersize rock by number of particles, or both. The size of any oversize rock shall allow for the placement of minimum of three layers of rock must be achieved when filling the 2.5 feet (0.76 m) high Terramesh units.









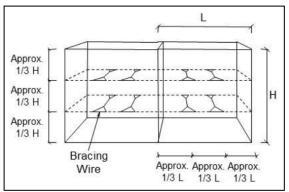


Fig. 17 - Bracing layout

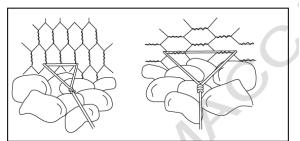


Fig. 18 – Detail on the mesh openings (minimum 2) to be braced







The cells shall be filled in stages so that local deformation may be avoided. That is, at no time shall any cell be filled to a depth exceeding 1-ft (0.30 m) higher than the adjoining cell. It is also recommended to slightly overfill the baskets by 1 to 2 in (25 to 50 mm) to allow for settlement of the rock. See Fig. 5.

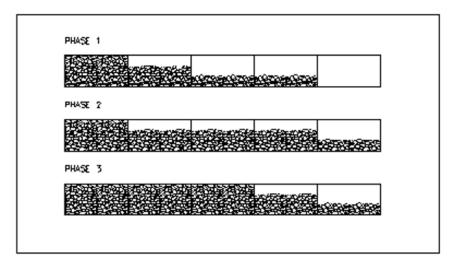


Fig. 5

## **BACKFILLING OPERATIONS**









## **BACKFILLING OPERATIONS**









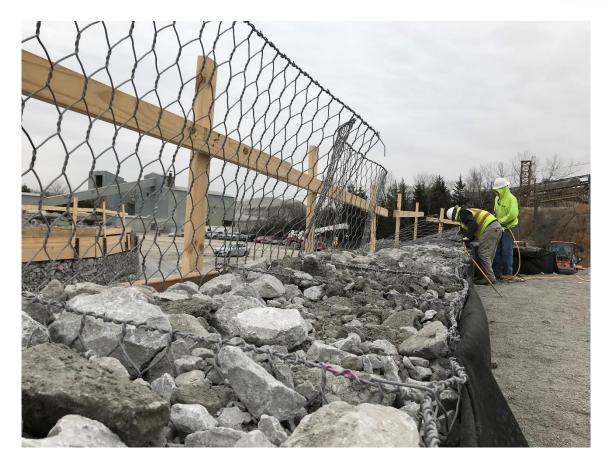












## READY FOR NEXT LAYER



ASCE



















UNIVERSITY OF NEBRASKA

OMAHA, NEBRASKA 68182

DEPARTMENT OF CIVIL ENGINEERING





38<sup>th</sup> ANNUAL GEOTECHNICAL SEMINAR

GEO-Omaha 2021

FEBRUARY 5, 2021 SCOTT CONFERENCE CENTER 6450 PINE STREET OMAHA. NEBRASKA

# Thank you!! Questions???

s.rignanese@maccaferri.com



Engineering a Better Solution