Foundation Performance of Millennium Tower in San Francisco, CA

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Acknowledgements

- Support from Mission Street Development during legal proceedings;
 my engagement ended in early 2020
- Slate Team: Nathaniel Wagner, Debra Murphy, many others
- Shannon & Wilson Team: Jeremy Butkovich, Hamid Nouri, others
- Ron Hamburger, SGH

Outline

- Millennium Tower: why it matters
- Site conditions
- MT structure and construction timelines
- Foundation movement mechanisms
- Foundation performance
- Simulations
- Status
- Lessons learned





Constructed 2005-2009 58 stories, 197 m (645 ft) tall

Tallest & most expensive residential tower in San Francisco
Views from the Sierra to the Cascades to the Farallon Islands
Most expensive unit sold in 2013 for \$13.5 M
Construction Cost - \$600 M Cumulative Sales - \$750 M

Courtesy: RO Hamburger

What happened?

2005: Groundbreaking

Predicted settlement 10-15 cm

2009: Tower & Podium construction completed

- Predicted settlement was exceeded in March 2008
- Settlement reached ~18 cm by early 2009
- Settlement estimates updated as part of permitting process

2010: Construction begins adjacent to Millennium Tower

2013: Last unit sold

Settlement 28 cm

2016: Legal proceedings initiated

Settlement 35 cm

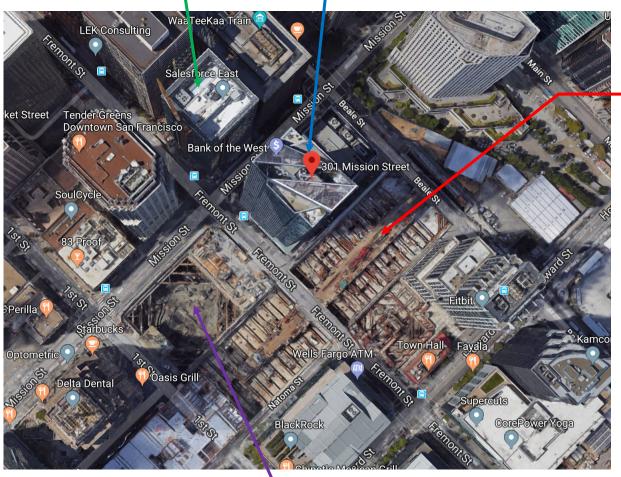
2018: Adjacent construction ends

Settlement 40 cm



Millennium Tower 2005-2009

Sales Force East 2013-2015 -



Transbay transit terminal 2010-2018

Courtesy: RO Hamburger

Salesforce Tower 2015-18 -

Why is this case history important?

Foundation design practice changed

- New high rises on San Francisco "infirm soils" now use piles to rock
- City of San Francisco now requires geotechnical peer review for high rise buildings

Learning opportunities

- What soil deformation mechanisms produced the movements?
- Could the movements have reasonably been anticipated?
- Secondary compression prediction and settlement mitigation

Are the recent changes in practice appropriate?

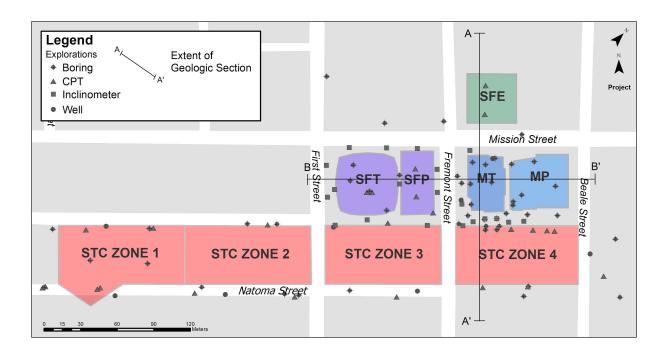


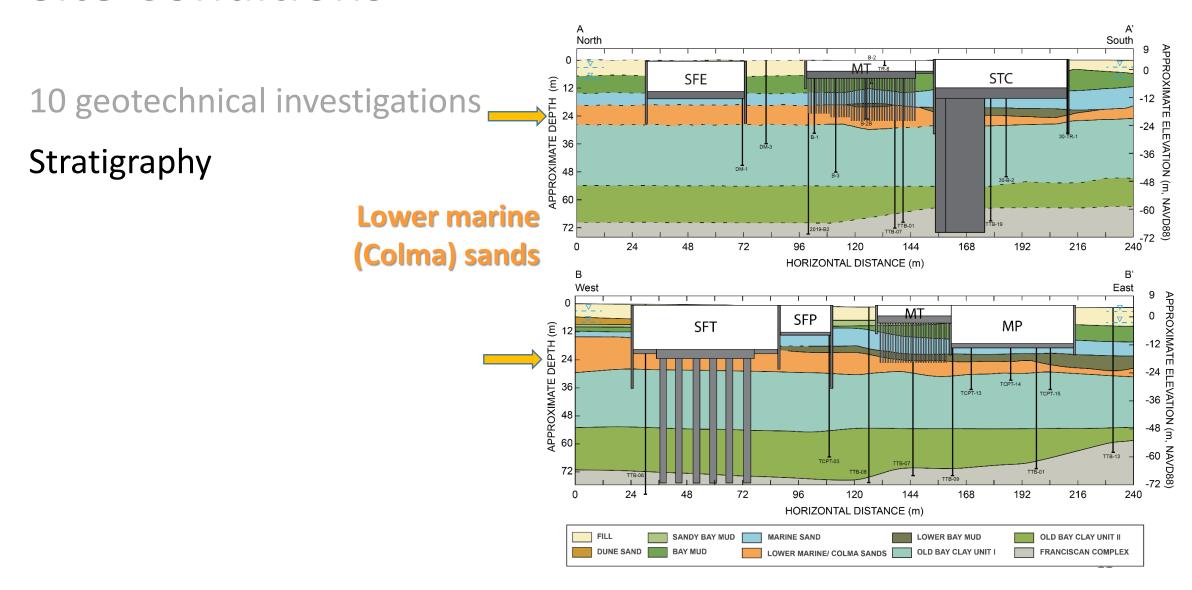
Outline

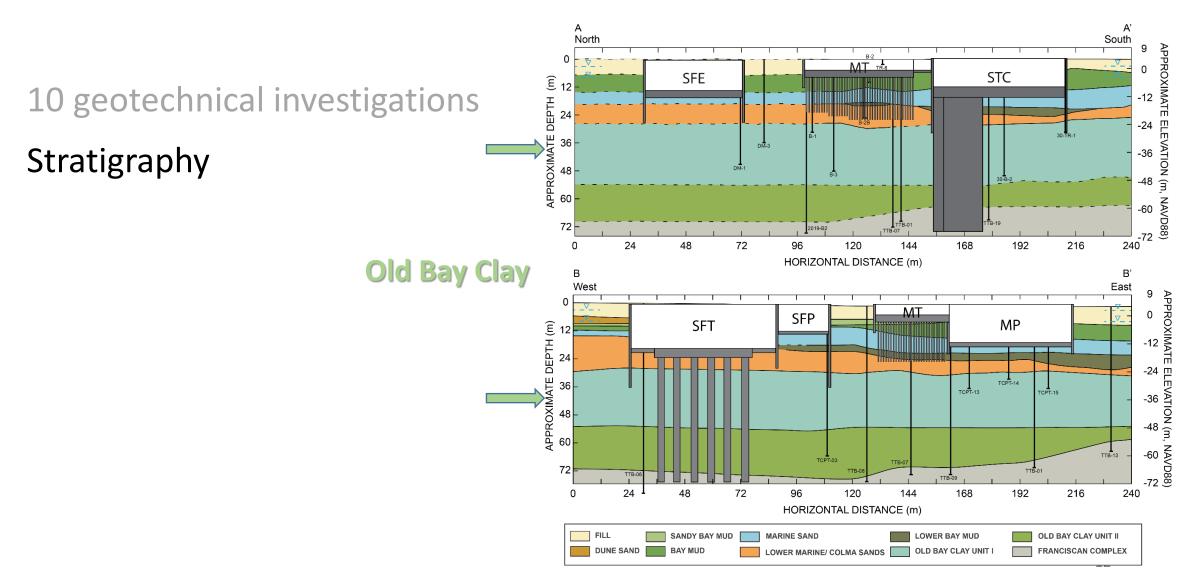
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10 geotechnical investigations



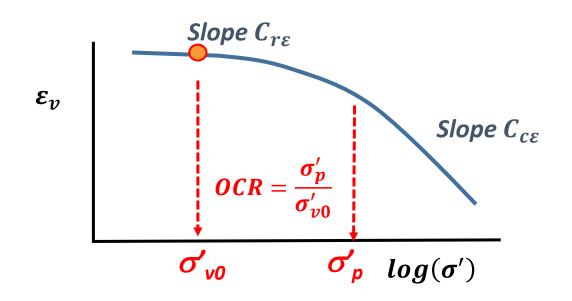


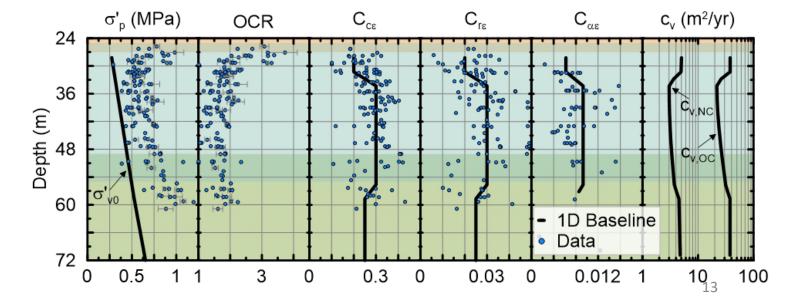


10 geotechnical investigations

Stratigraphy

Properties of Old Bay Clay

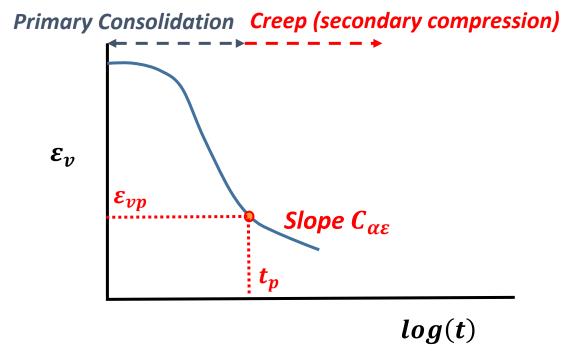




10 geotechnical investigations Stratigraphy

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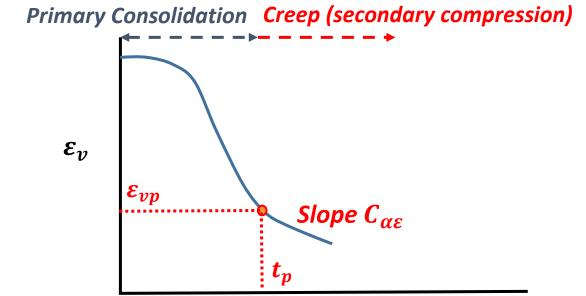
Secondary compression properties

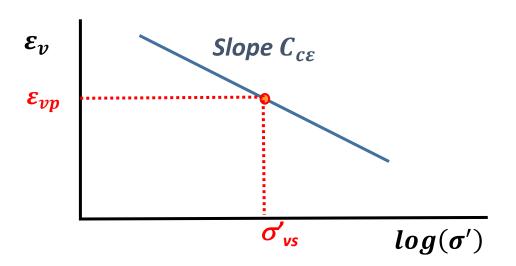


10 geotechnical investigations
Stratigraphy

Properties of Old Bay Clay

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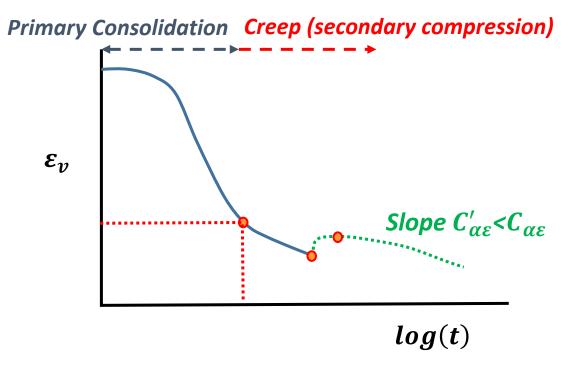


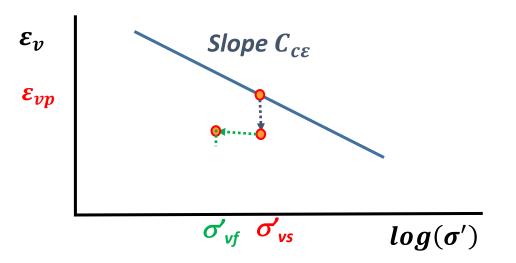
log(t)

10 geotechnical investigations
Stratigraphy

Properties of Old Bay Clay

Secondary compression properties (Wagner et al. 2021)



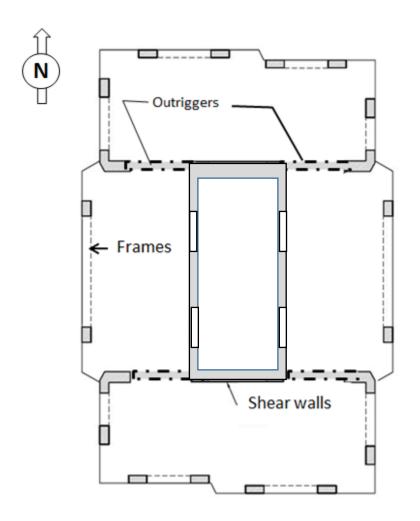


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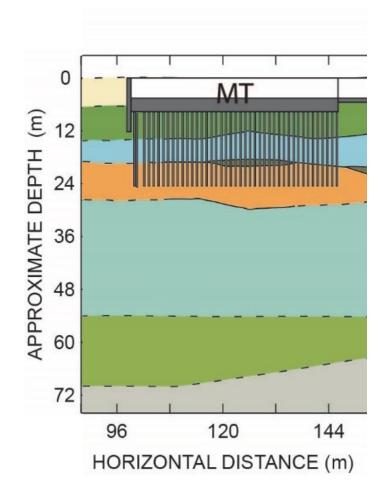
MT structural system



Courtesy: RO Hamburger

MT structural system

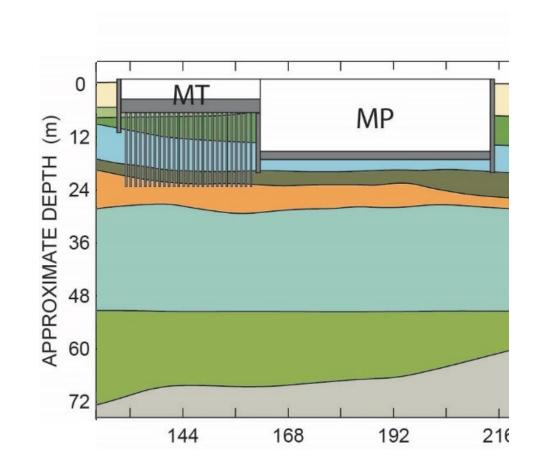
Sub-structure: one level basement, 4.6 m, 3 m pile supported mat



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Podium east of MT

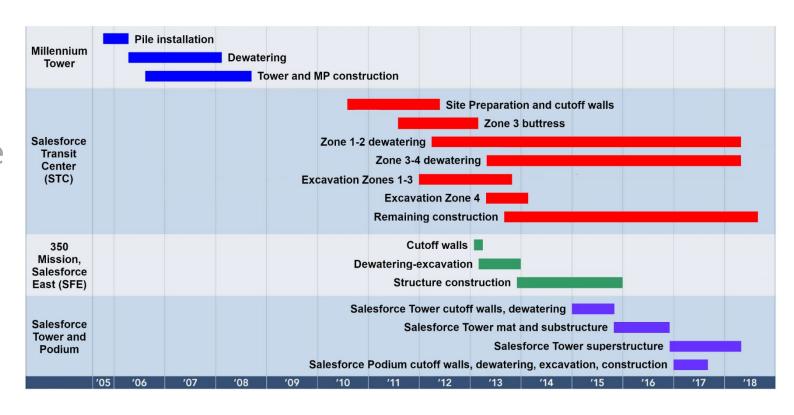


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Construction timelines

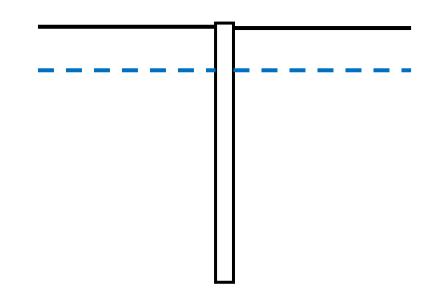


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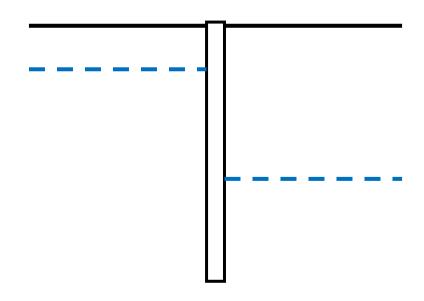


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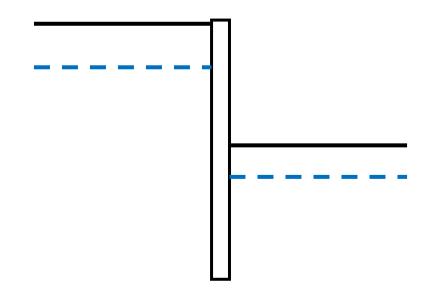


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Construction timelines



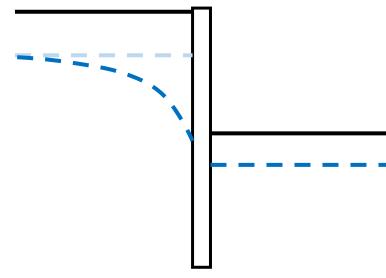
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Podium east of MT

Construction timelines

Cutoff walls; gw lowering possible if:



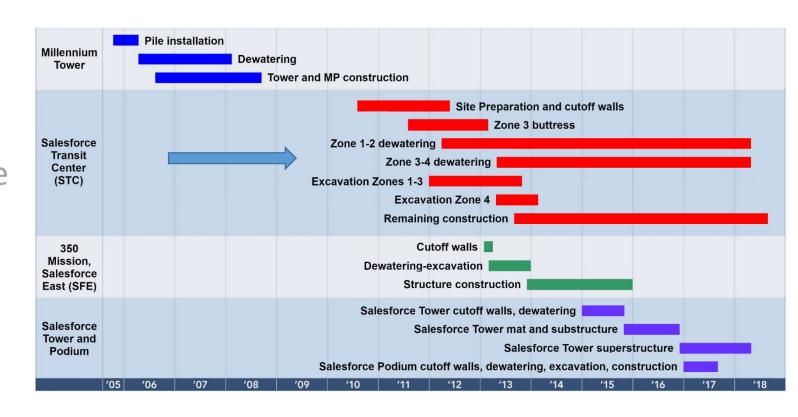
Leaks through walls
Gaps in walls
Underseepage

MT structural system

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Podium east of MT

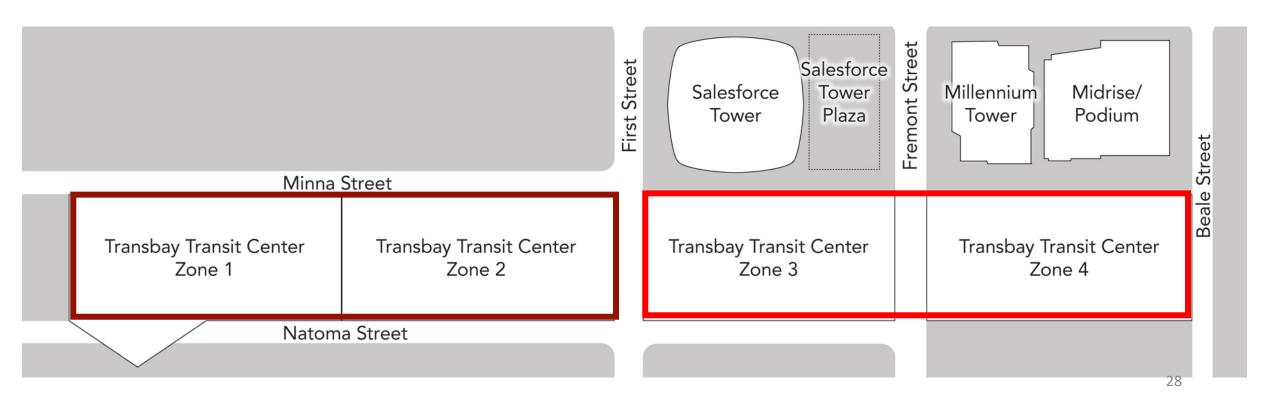
Construction timelines



Transit center constructed in four "zones"

Zones 1-2 dewatering began in 2012

Zones 3-4 dewatering began in 2013



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Volume Change

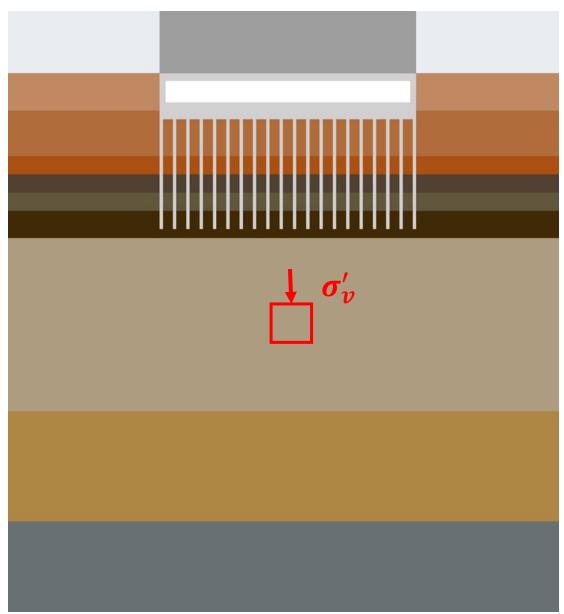
Effective stress on soil element:

$$\sigma_v' = \sigma_{v0} + \Delta \sigma - u$$

Tower construction causes $\Delta \sigma$ increase

Lowering of groundwater decreases u

Both increase σ'_v , inducing consolidation



Volume Change

Effective stress on soil element:

$$\sigma_v' = \sigma_{v0} + \Delta \sigma - u$$

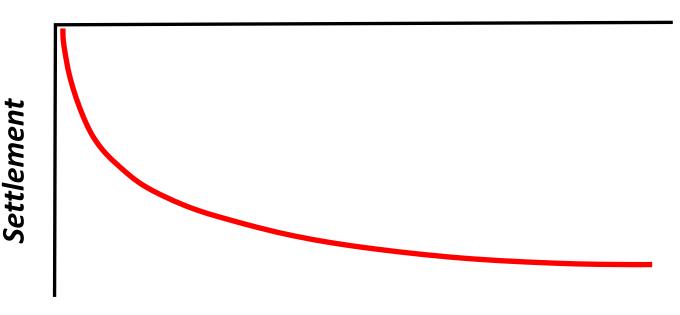
Tower construction causes $\Delta \sigma$

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Both increase σ'_v , inducing consolidation

Settlement rate fast initially, then slows with time

Time Since Stress Increase



Shear Deformation of Braced Excavations

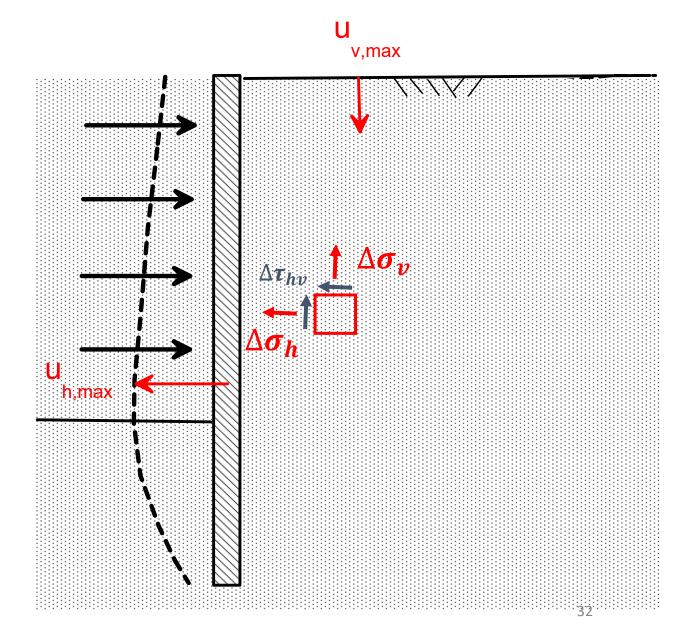
Supported excavations change stresses in retained soils

Wall movement inwards

Ground settlement behind wall

Produces tilt of structures towards excavation

e.g., Peck 1969; Clough et al. 1979; O'Rourke 1976, 1981; Hashash and Whittle 1996; Moorman 2004; Konda et al. 2010; Korff et al. 2016



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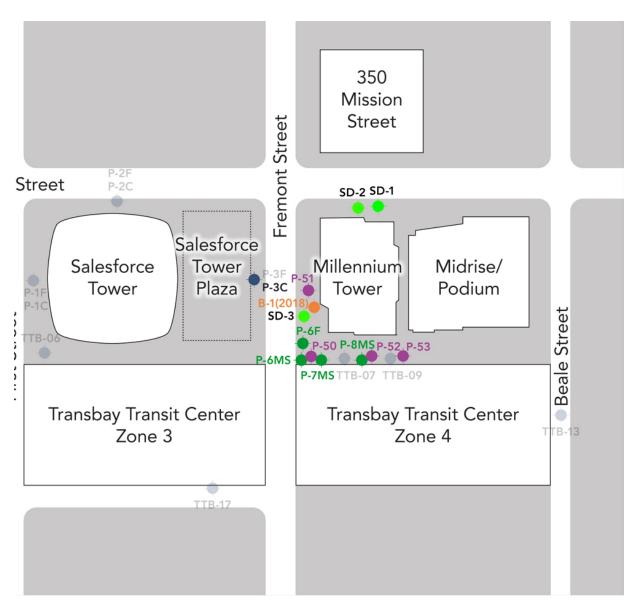
Foundation Performance

- Available instrumentation/data
- Interpretation



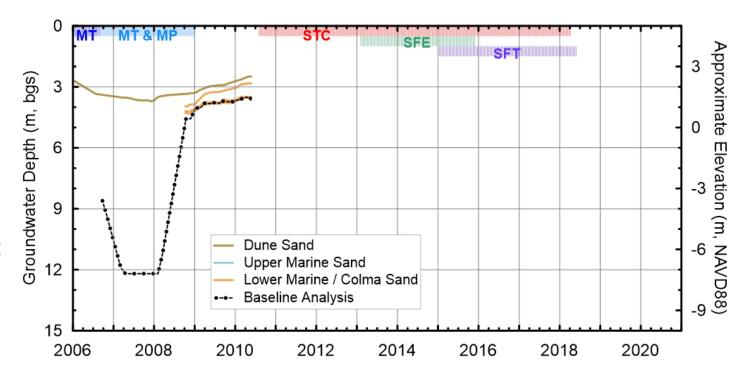
Piezometers in Colma

 Selected locations are outside of excavations & near MT



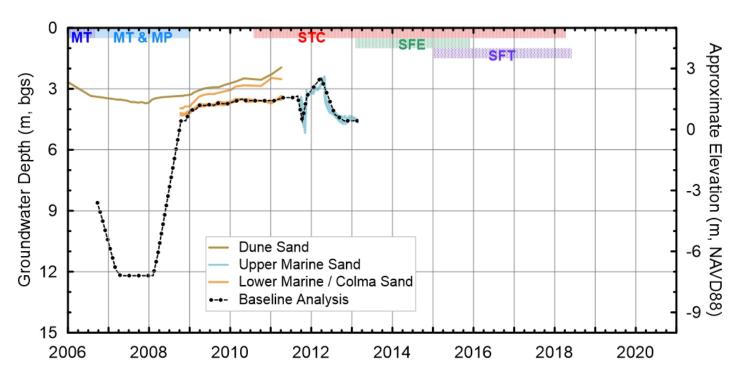
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- Pre-construction gwt at ~3 m
- 2007-08: podium construction; rapid recovery



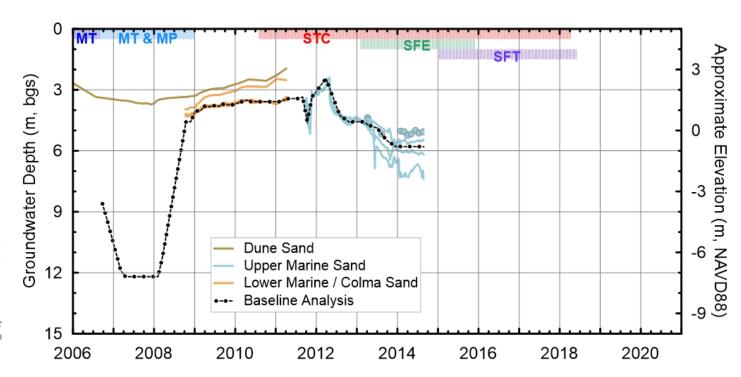
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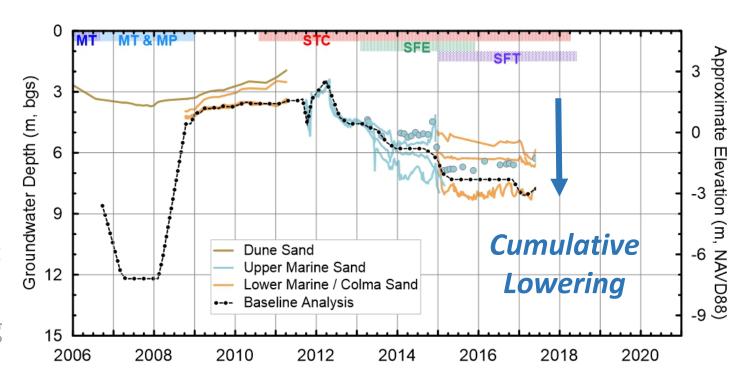
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Water Pressure

Piezometers in Colma

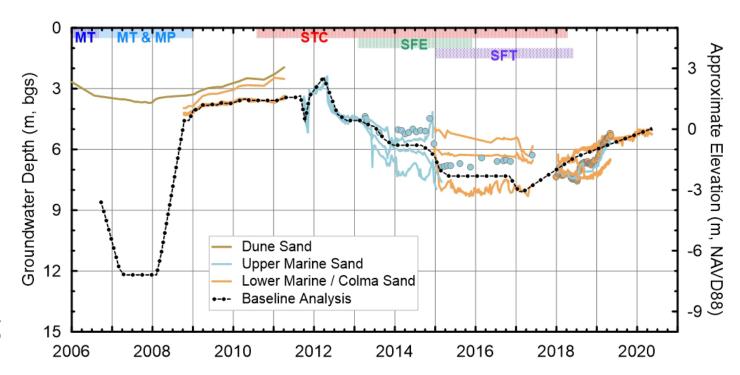
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- 2015-17: SFT dewatering



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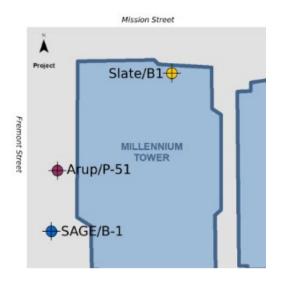
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- > 2018: recovery

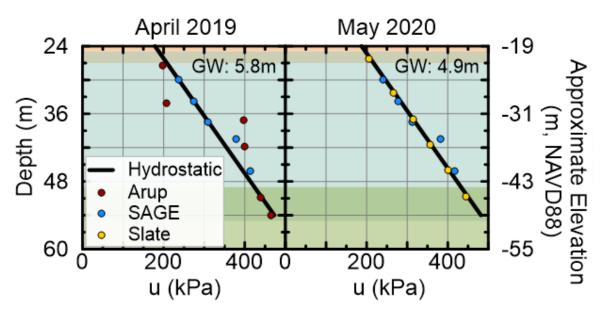


Water Pressure

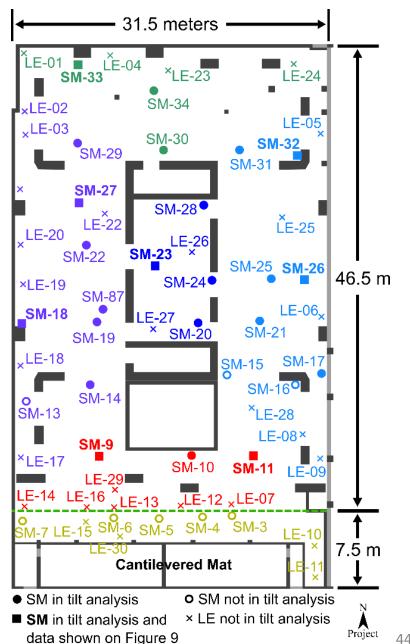
Piezometers in OBC

- Used to evaluate potential under-consolidation
- Interpretation: Consolidation effectively complete in mid-2020 (maybe earlier)

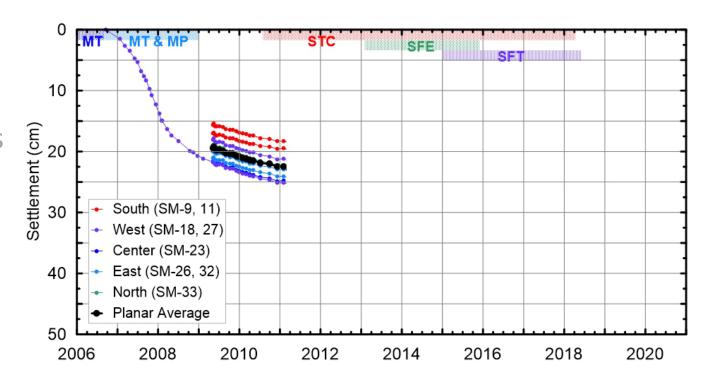




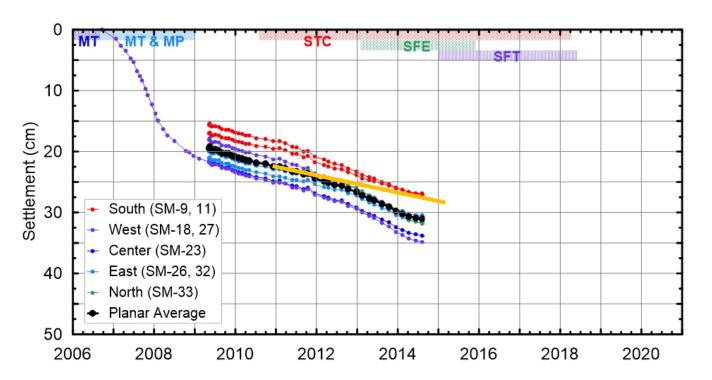
- 2006-09: single monument on 1st floor
- May '09 present: 31 markers



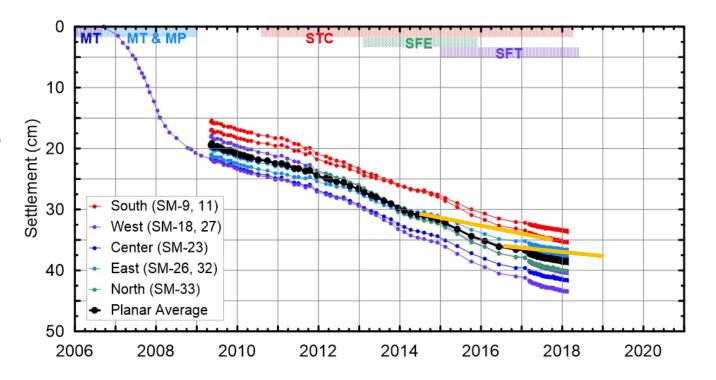
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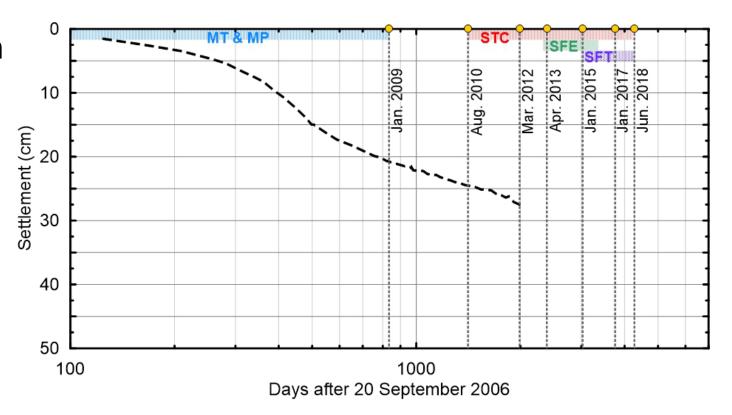
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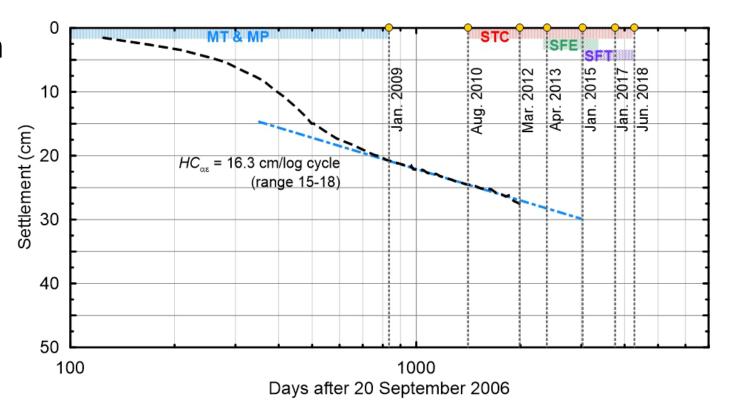
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Prior to adjacent construction

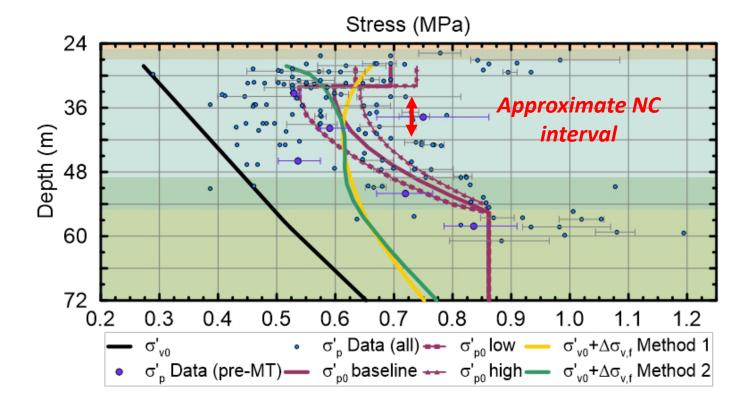


Prior to adjacent construction



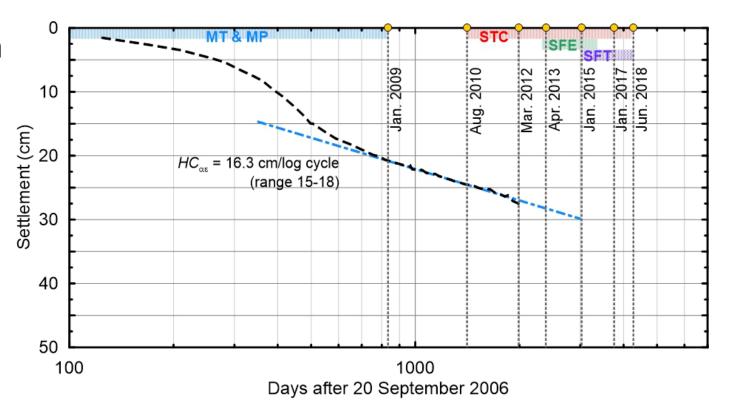
Prior to adjacent construction

• Stress comparison to σ_p'



Prior to adjacent construction

- Stress comparison to σ_p'
- Secondary compression was strong contributor
- Some consolidation may have still been occurring > 2009

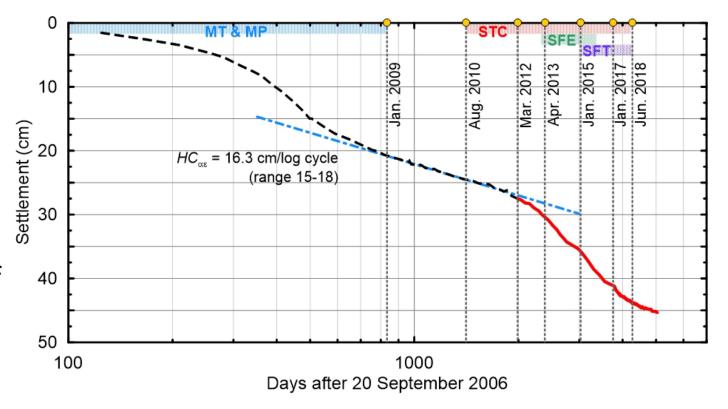


H=7-12 m \rightarrow implied $C_{\alpha\varepsilon} = 0.013 - 0.022$

Laboratory data: 0.006-0.016 (avg 0.01)

Prior to adjacent construction

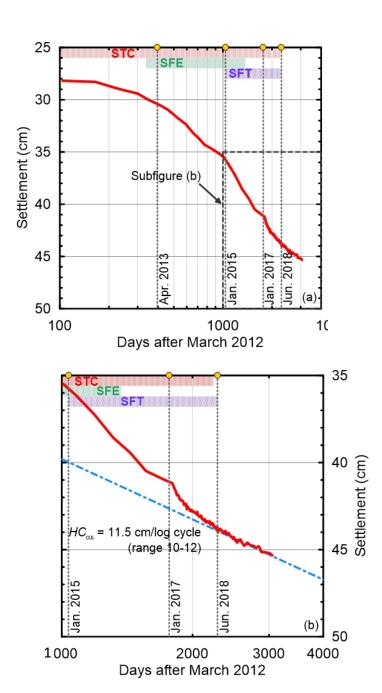
- Stress comparison to σ_p'
- Secondary compression was strong contributor
- Some consolidation may have still been occurring
- Acceleration following onset of adjacent construction



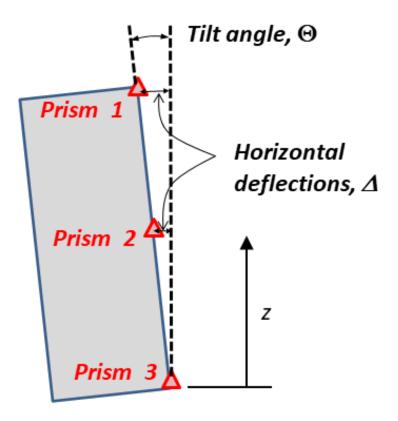
Prior to adjacent construction

Since 2012

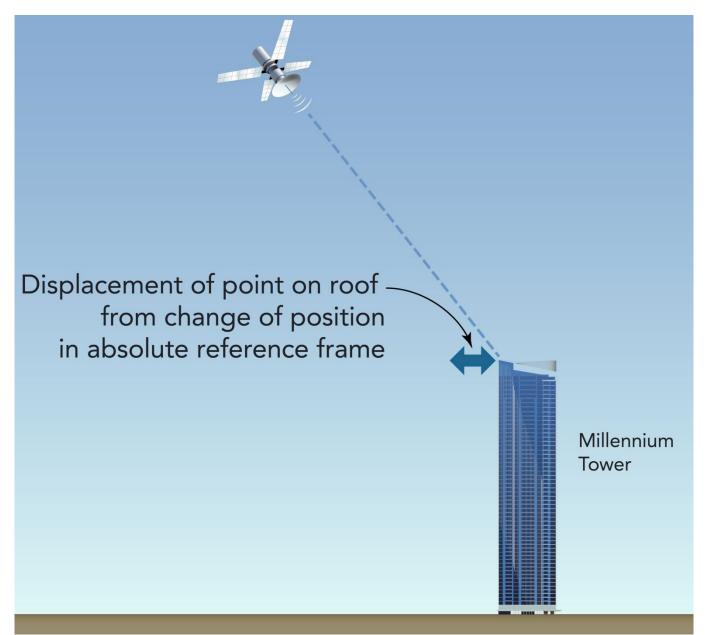
- Concept of time "reset"
- Reset in 2012
- Pattern complicated by multiple loading cycles
- Recent slope suggests consolidation is complete



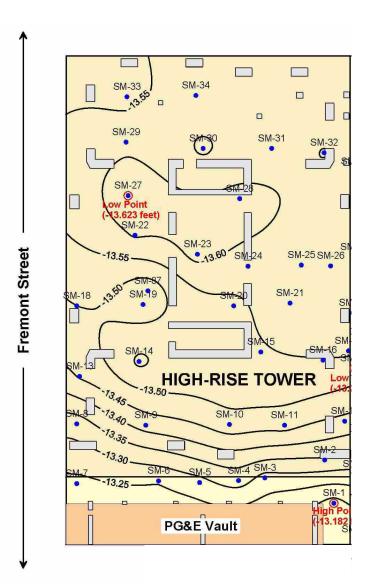
Prisms (Jan '17 – present)



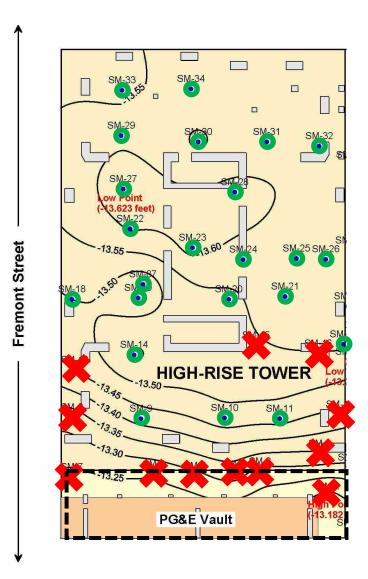
- Prisms (Jan '17 present)
- InSAR data (May '09 Mar '17)



- Prisms (Jan '17 present)
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- Settlement markers (May '09 present)



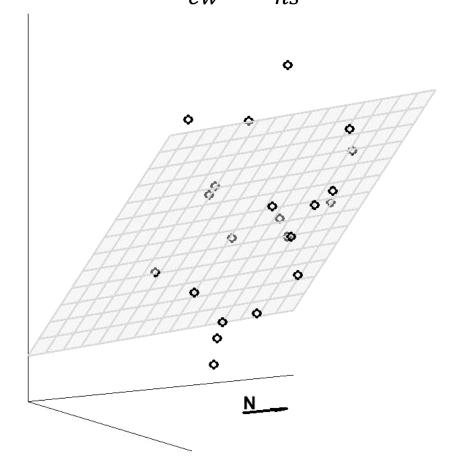
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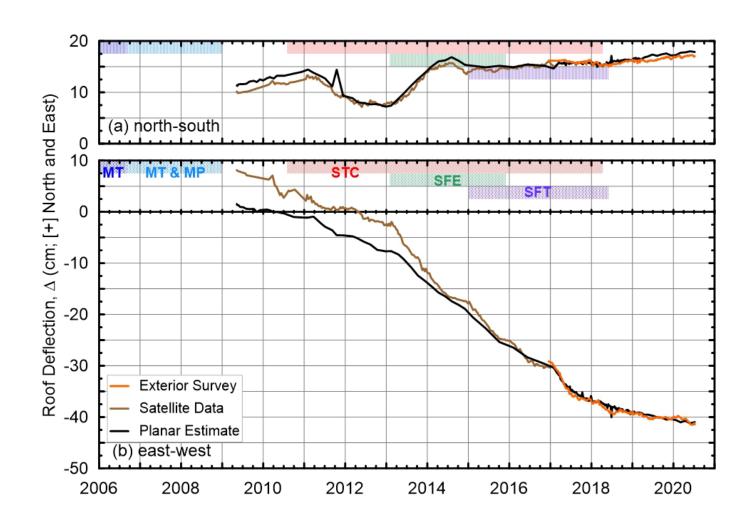


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- Settlement markers (May '09 present)

Planar fit

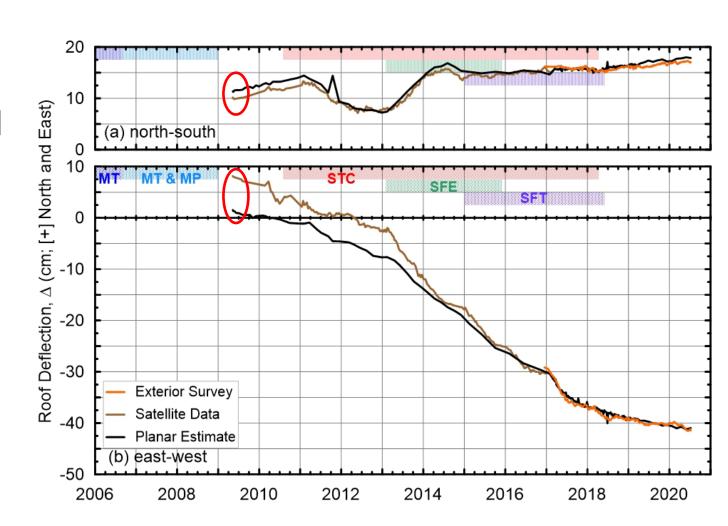
Angles from horizontal: $\theta_{ew} \& \theta_{ns}$





Initial (May '09) tilts estimated

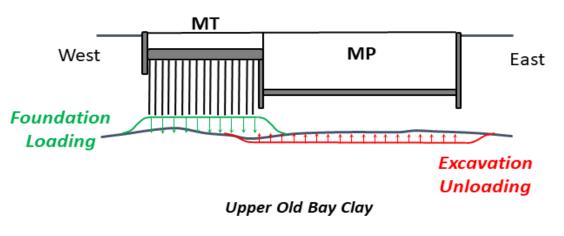
Why initial tilt to east?

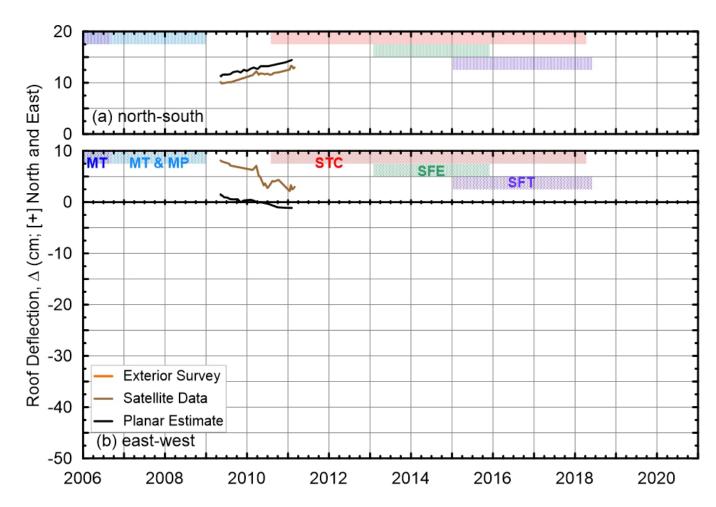


Initial (May '09) tilts estimated

2009-11: movement to north and west

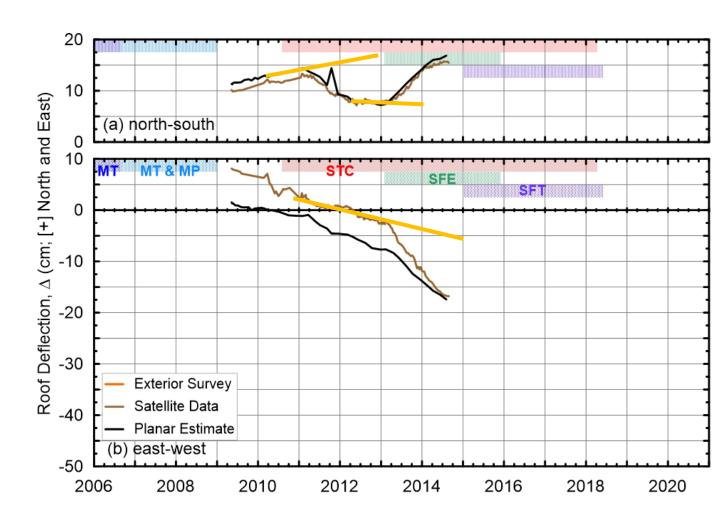
Why the west trend in tilt?





Initial (May '09) tilts estimated 2009-11: movement to north and west

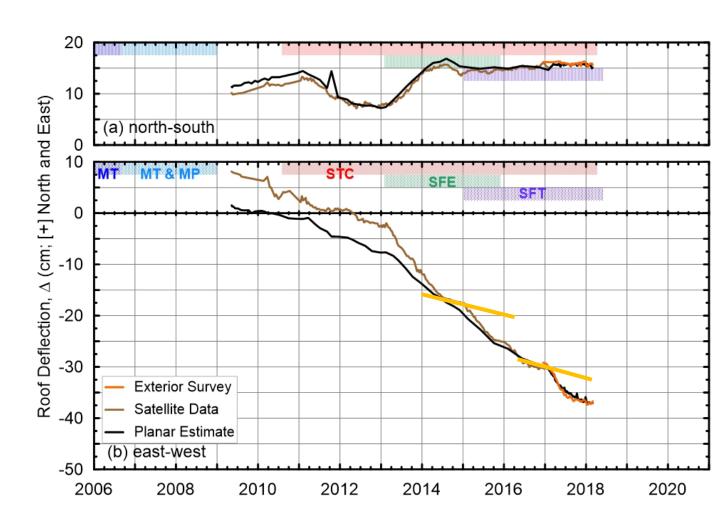
2012-13: STC dewatering & excavation. 2013 SFE



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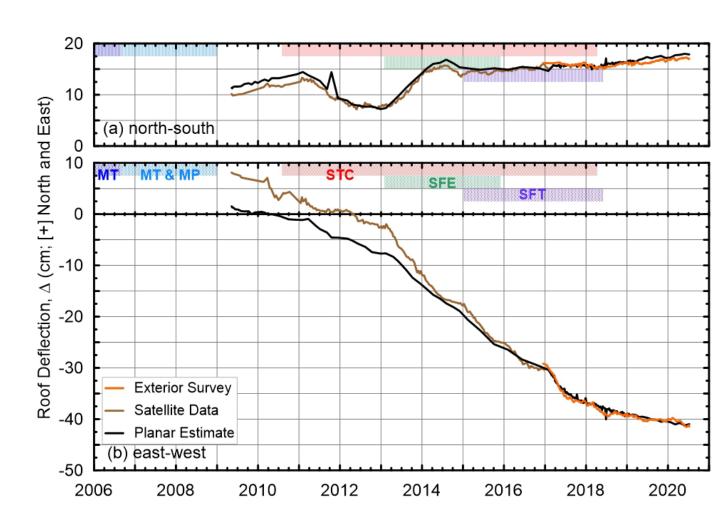


Initial (May '09) tilts estimated 2009-11: movement to north and west

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After 2018: deceleration of W tilt



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Simulations

- 1D & 3D
- Main objectives:
 - Are observed foundation movements predicable?
 - Relative significance of volume change and shear deformation on settlements

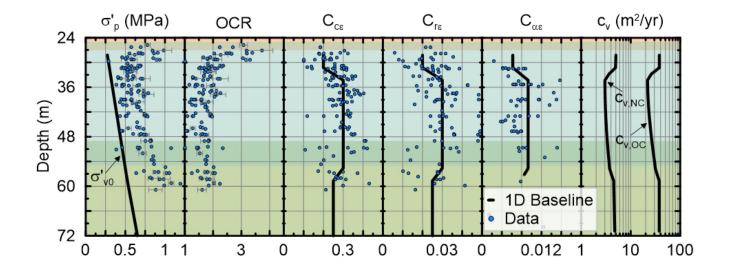


1D Settlement Predictions

Finite difference solution of Terzaghi (1925) diffusion PDE:

- Time-dependent: loading, u, c_v
- Creep considered during and after primary consolidation

Baseline soil properties and their variability



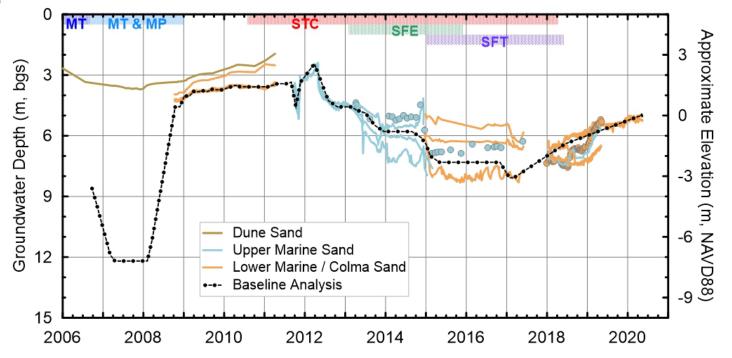
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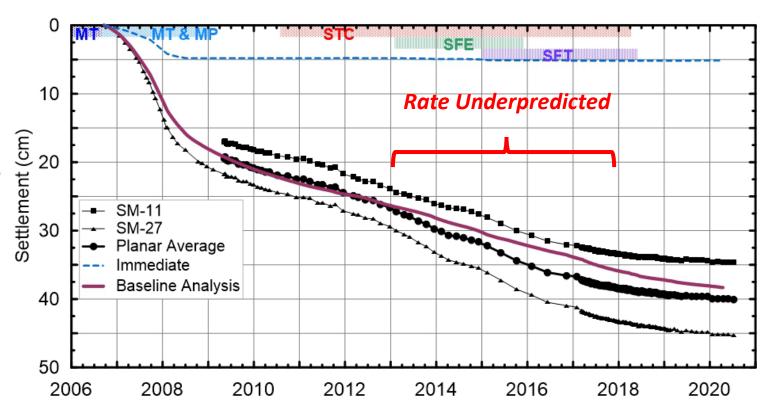
Adjacent construction considered through gw variations in time



1D Settlement Results

Baseline properties

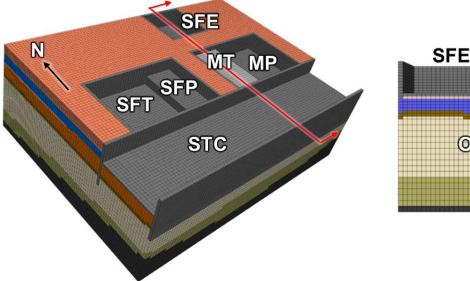
- U_{1D} =81% (late 2011)
- U_{1D}=96% (late 4/2019)
- OCR = 1.02 in 5/2020 due to gw rebound (2.1 m)

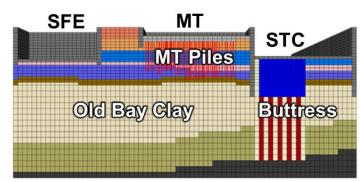


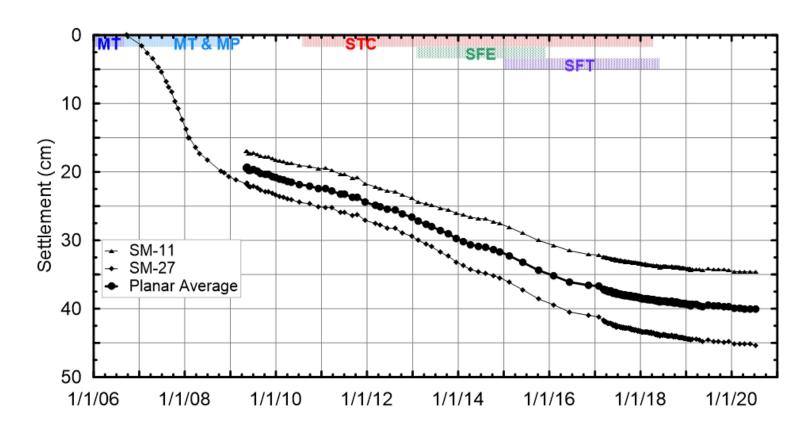
3D Settlement Predictions

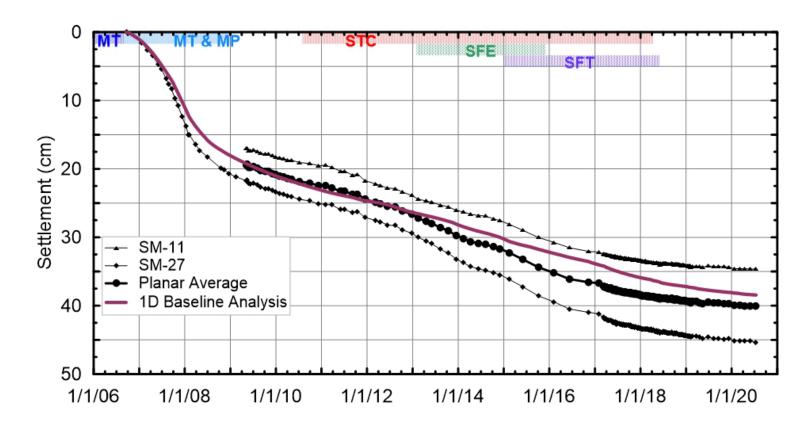
FLAC3D Ver 7

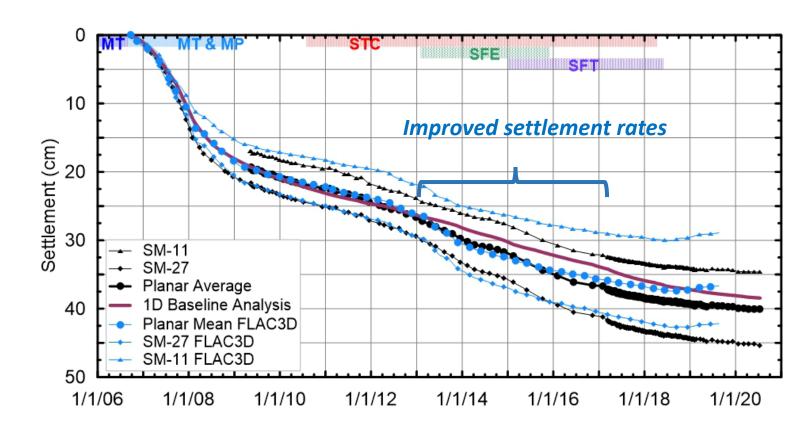
Same properties as 1D, but variable stratigraphy
Special calibration of soft soil creep model



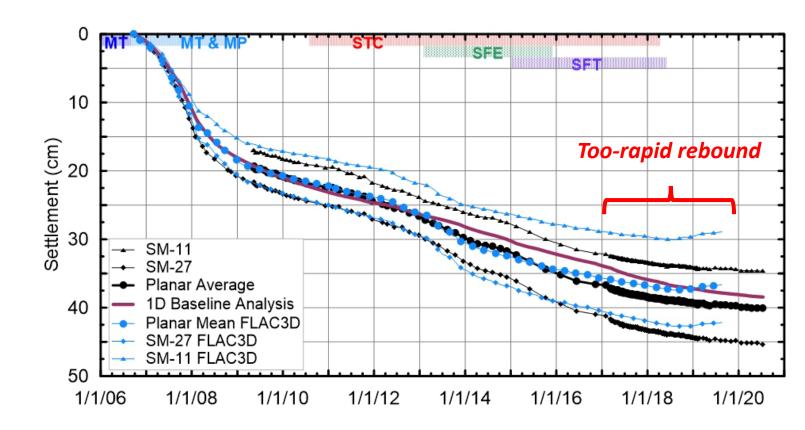








- U_{3D} =90% (late 2011)
- U_{3D}=100% (late 4/2019)



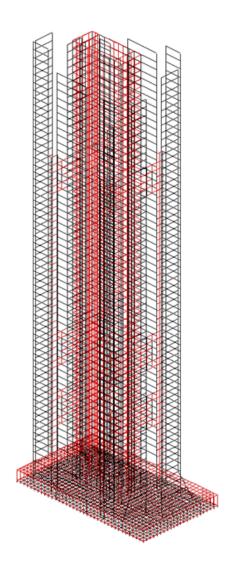
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Structural analysis (SGH):

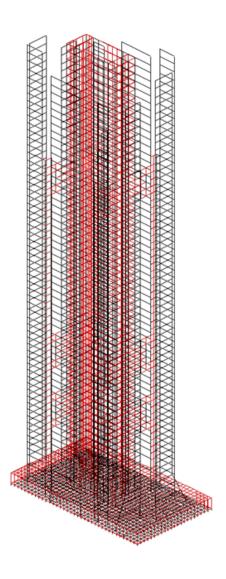
 Tower "seismic response considering settlement is essentially the same as that neglecting it"



Courtesy RO Hamburger

Structural analysis (SGH):

Retrofit not required

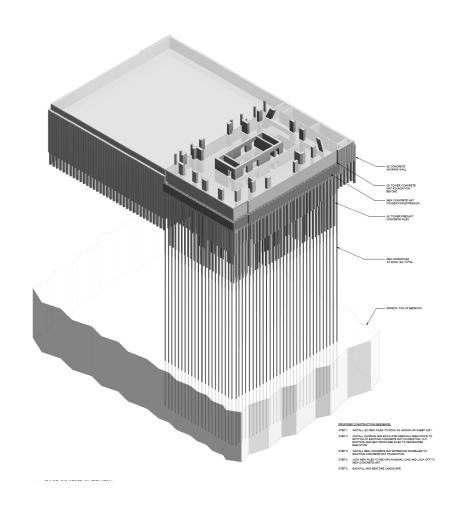


Courtesy RO Hamburger

Structural analysis (SGH): Retrofit not required

Retrofit underway to address stigma-related losses

Perimeter piles (N and W sides)

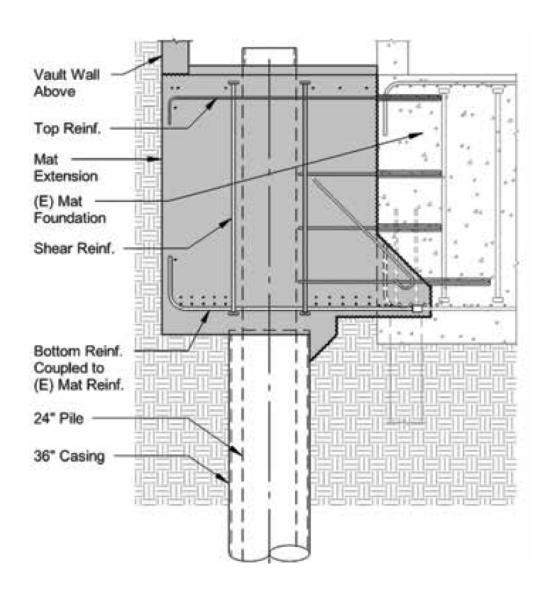


Courtesy RO Hamburger

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Retrofit underway to address stigma-related losses

- Perimeter piles (N and W sides)
- Mat extension

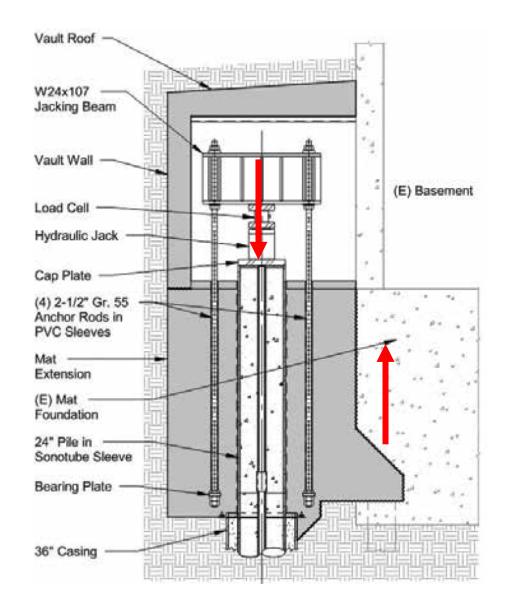


Hamburger et al. (2021)

Structural analysis (SGH): Retrofit not required

Retrofit underway to address stigma-related losses

- Perimeter piles (N and W sides)
- Mat extension
- Jack piles, to transfer 18% of building load (later reduced)



Hamburger et al. (2021)

Outline

- Millennium Tower: why it matters
- Site conditions
- MT structure and construction timelines
- Foundation movement mechanisms
- Foundation performance
- Simulations
- Status
- Lessons learned



Lessons Learned

Be cautious in application of precedent

Multiple consequential episodes of volumetric and shear deformation in OBC layer

MT foundation sensitive to stress perturbations in OBC due to its nearly normally consolidated state over ~ 10 m depth interval

Volumetric deformations are the dominant contributor to settlements

Original and re-activated deformations are predictable, : could be anticipated during design and mitigated

More Information

<u>Secondary compression</u>: Wagner, N, M Largent, JP Stewart, C Beyzaei, D Murphy, J Butkovich, JA Egan (2021). <u>Stress history-dependent secondary compression of San Francisco Bay region Old Bay Clays</u>, *J. Geotech. Geoenv. Eng.*, **147**, 04021045.

<u>Case history</u>: Stewart, JP, N Wagner, D Murphy, J Butkovich, M Saqui, H Nouri, H Curran, D Maffioli, JA Egan (forthcoming). Foundation settlement and tilt of Millennium Tower in San Francisco, California, *J. Geotech. Geoenv. Eng.*, https://doi.org/10.1061/JGGEFK/GTENG-10244.